

1 Identification of potential sewer mining locations: a Monte- 2 Carlo based approach

3 I. K. Tsoukalas*, C. K. Makropoulos* and S. N. Michas**

4
5 * Department of Water Resources and Environmental Engineering, National Technical
6 University of Athens, Heroon Polytechneiou 5, GR-15780, Zographou, Greece

7 (e-mail: itsoukal@mail.ntua.gr; cmakro@mail.ntua.gr)

8 ** Hydroexigiantiki Consultants Engineers, 3 Evias str, GR-15125, Marousi, Greece

9 (e-mail: smichas@hydroex.gr)

10 11 **Abstract**

12 Rapid urbanization affecting demand patterns, coupled with potential water shortages due to
13 supply side impacts of climatic changes have led to the emergence of new technologies for
14 water and wastewater reuse. Sewer mining is a novel decentralized option that could
15 potentially provide non-potable water for urban uses, including for example the irrigation of
16 urban green spaces, providing a mid-scale solution to effective wastewater reuse. Sewer
17 mining is based on extracting wastewater from local sewers, treat at the point of demand and
18 entails in some cases the return of treatment residuals back to the sewer system. Several
19 challenges are currently in the way of such applications in Europe, including public perception,
20 inadequate regulatory frameworks as well as engineering issues. In this paper we consider
21 some of these engineering challenges, looking at the sewer network as a system where multiple
22 physical, biological and chemical processes take place. We argue that prior to implementing
23 sewer mining, the dynamics of the sewer system should be investigated in order to identify
24 optimum ways of deploying sewer mining without endangering the reliability of the system.

25 Specifically, both wastewater extraction and sludge return could result in altering the
26 biochemical process of the network, thus unintentionally leading to degradation of the sewer
27 infrastructure. We propose a novel Monte-Carlo based method that takes into account both
28 spatial properties and water demand characteristics of a given area of sewer mining
29 deployment while simultaneously accounts for the variability of sewer network dynamics in
30 order to identify potential locations for sewer mining implementation. The outcomes of this
31 study suggest that the method can provide rational results and useful guidelines for upscale
32 sewer mining technologies at a city level.

33 **Keywords**

34 Decentralized wastewater options; hydrogen sulphide; Monte-Carlo method; sewer mining;
35 upscaling

36 **1 Introduction**

37 Rapid urbanization and potential water shortages due to, inter alia, climatic variability have led
38 to the emergence of new technologies in water and wastewater reuse aiming to provide
39 alternative water sources for more resilient cities. Sewer mining (SM) is such technology, which
40 is based on extracting wastewater from local sewers for reuse applications (after treatment)
41 and (often) returning treatment residuals (sludge) back to the system (e.g., Sydney Water,
42 2008). Typical uses of this recycled water are industrial cleaning and cooling, as well as,
43 irrigation of urban green spaces (Hadzihalilovic, 2009; Marleni et al., 2012). Literature classifies
44 this technology as a decentralized option (Makropoulos et al., 2017) because it is applicable
45 (and suitable) at a development level (for example, up to 5 000 households). This is also
46 highlighted in Marleni et al., (2012) where it is argued that this practice is not intended for
47 individual use (indoor appliance) rather than for collective/cluster scale developments.
48 Furthermore, the latter authors remark that usually such systems are not managed by central
49 water utilities (or governmental organizations) but by private establishments under some

50 license agreements. As such, sewer mining is a promising reuse option that lies in the interplay
51 between reuse at household scale (e.g., grey water reuse; cf. Liu (2010) and Makropoulos and
52 Butler (2010)) and centralized reuse at the wastewater treatment plant (WTP) level
53 (Andreadakis et al., 2006). Current sewer mining projects mostly involve park and sports fields'
54 irrigation. Most of them are operating in Australia (Sydney Water, 2009) where the climate is
55 dry and water should be treated carefully. It is worth highlighting that in most cases the treated
56 water is used for non-drinking purposes. Despite public perception, concerns and inadequate
57 regulatory frameworks that may raise potential barriers for sewer mining implementation,
58 there are engineering issues that have to be addressed. A sewer network is a system where
59 multiple physical, biological and chemical processes take place (Pomeroy, 1990). Hence, prior
60 implementing sewer mining, the dynamics of the system should be investigated in order to
61 identify optimum ways of deploying sewer mining without endangering the reliability of the
62 system. Specifically, both wastewater extraction and sludge return could result in altering the
63 biochemical process of the network, thus unintentionally leading to degradation of the
64 infrastructure. In this paper we focus on addressing some of the engineering challenges linked
65 to the potential deployment of such technologies at the city scale. Typical engineering issues
66 associated with sewer mining are odour and corrosion. Both of them are related to the
67 production of hydrogen sulphide (H_2S) in sewer pipes. This study focuses on identifying
68 possible locations for SM placement subject to minimizing H_2S build-up.

69 **2 Materials and methods**

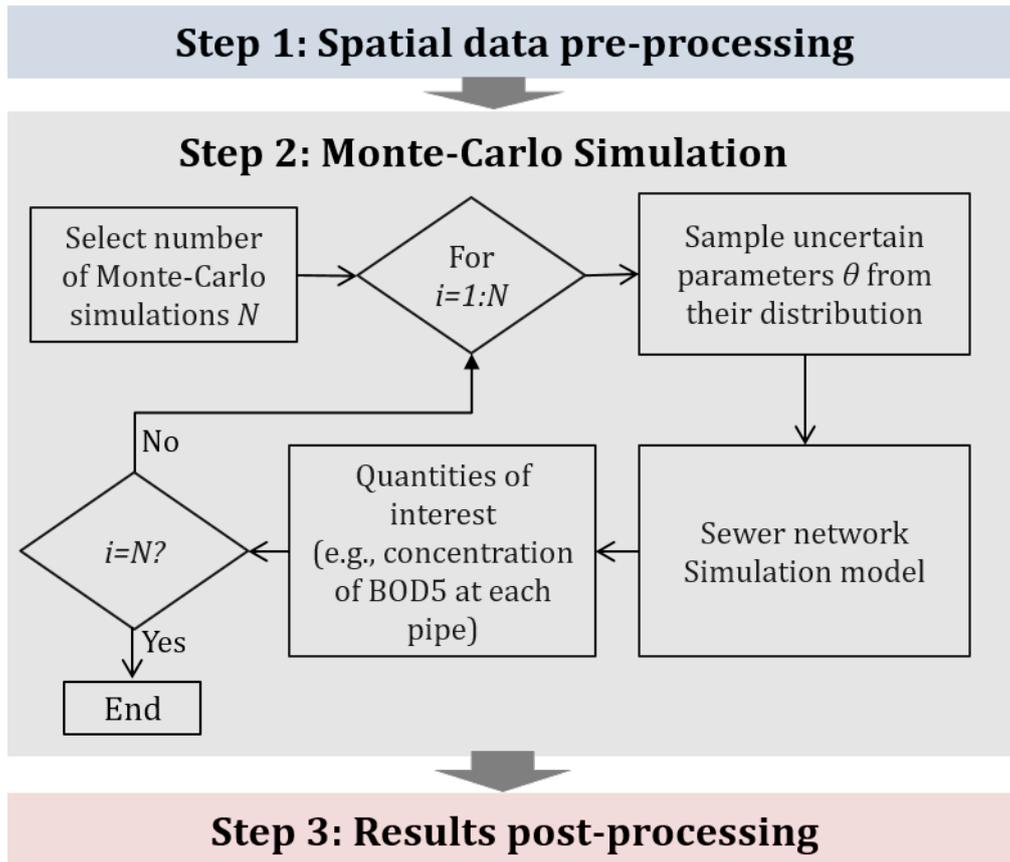
70 **2.1 Methodology Description**

71 While trying to address this issue i.e., SM placement by taking also into consideration H_2S
72 production, we propose a methodology consisting of three steps, (I) a spatial data pre-
73 processing step during of which the spatial properties and water demand characteristics are
74 being identified (II) a Monte-Carlo simulation (MCS) step, which involves the simulation of the

75 sewer network in order to account for the variability of sewage discharge into the network and
76 finally, (III) a post-processing step which comprises (III-a) the definition of appropriate metrics
77 that quantify the output of interest and (III-b) a multi-criteria analysis of the results. A
78 schematic description of the proposed methodology is given in Figure 1. During the first step
79 the available spatial information (i.e., sewage network topology and assets, topography, water
80 and land uses) is imported into the procedure in order to identify land uses that will benefit
81 from sewer mining (in our case green areas and parks). It involves a procedure of locating
82 neighbouring sewer network components (e.g., nodes) which are close to areas of interest. In
83 more detail, this is done by delineating a wider area surrounding the original one (e.g., add 10 m
84 offset to green areas) and subsequently identifying the nodes that lie into those wider areas.
85 Finally, the paths from the identified nodes to an “*exit*” node are identified and stored. The exit
86 node could be a WTP or a node that links the understudy network with a broader larger
87 network. It is worth noticing that this path is unique for each node due to the “collective nature”
88 of sewer networks. The purpose of the second step is to propagate uncertainties related with
89 the input parameters to the quantities of interest (e.g., BOD₅ concentration or flow of each pipe).
90 Furthermore, the use of Monte-Carlo simulation allows the use of probabilistic functions and
91 metrics, which in-turn provide uncertainty-aware outputs. Typical examples of uncertain
92 parameters are the daily water consumption, daily and hourly variation coefficients of
93 wastewater discharge and BOD₅ loading (in terms of g/cap). Alternatively, one could use a
94 similar scenario-based approach to sample those parameters; (or in conjunction with MCS) in
95 order to investigate the effect of certain predefined scenarios (e.g., worst, base, favorable
96 conditions).

97 The third and final step involves the definition and the use of metrics i.e., utility functions or
98 risk functions that quantify the output of interest, in our case H₂S build-up, for a chain of pipes
99 (the paths specified in step I). We remark that BOD₅ can be directly associated with H₂S through

100 empirically derived relationships (e.g., Lahav et al., 2006; Marleni et al., 2015). Furthermore, as
 101 a final procedure, we use multi-criteria analysis which eventually leads to derivation of a Pareto
 102 front (based on conflicting criteria – e.g., suitability of location and green area water demand),
 103 which includes all the potential locations for sewer mining.



104

105 Figure 1: Overall methodological framework for the identification of potential SM locations.

106 2.2 Implementation Details

107 The involved MCS (step II) of the proposed procedure requires the use of a simulation model in
 108 order to calculate the hydraulic outputs of interest. While any simulation model can be
 109 employed (e.g., SWMM 5.0), in this study we employed a steady state simulation model which
 110 uses the typical hydraulic equations for sewer networks as described in Koutsoyiannis, (2011).
 111 The total design discharge Q_D which is used to assess the performance of the network is
 112 calculated as the sum of sewage discharge (Q_s) and dry weather flow (Q_{DWF}). The sewage
 113 discharge can be calculated as follows:

$$Q_s = q \times E \times \lambda_L \times \lambda_S \times \lambda_1 \times \lambda_2 / 86400 \text{ (m}^3/\text{s)} \quad (1)$$

114 Where, q is the indicative daily water consumption per capita (lpd), E is the serviced population,
 115 λ_L , is a loss coefficient of water distribution network, λ_S is a coefficient that express the
 116 percentage of water that stems to the sewage network, λ_1 , is a seasonal coefficient and λ_2 , is a
 117 coefficient of peak discharge. The dry weather flow can be calculated as follows:

$$Q_{DWF} = \lambda_{DWF} \times Q_s / \lambda_2 \text{ (m}^3/\text{s)} \quad (2)$$

118 Where, λ_{DWF} is a dry weather flow coefficient (typically set to 0.2). Although, in this study we
 119 use eq. (2) in order to align with information available from previous studies, it is worth
 120 mentioning that literature (cf., Koutsoyiannis, 2011) includes a variety of formulas for the
 121 calculation of the aforementioned quantity.

122 In order to assess the extent of H_2S , we decided to employ a simple qualitative indicator known
 123 as the "Z formula" (US EPA Sulphide Control Manual 6). The dimensionless metric Z was
 124 originally proposed by Bielecki & Schremmer, (1987) and Pomeroy, (1990) for a single pipe i
 125 in order to quantify the probability of H_2S build-up. It is expressed as follows:

$$Z_i = \frac{0.3 \times 1.07^{T-20} \times [BOD_5]_i}{J_i^{0.5} \times Q_i^{1/3}} \times \frac{P_i}{b_i} \quad (3)$$

126 where, i is the pipe index, T is the sewage temperature ($^{\circ}C$), $[BOD_5]_i$ is the concentration of
 127 Biochemical Oxygen Demand of 5 days (mg/l), J_i is the pipe slope, Q_i is the discharge (m^3/s), P_i
 128 is the wetted perimeter of the pipe wall (m) and b_i the surface width (m) of the stream. It is
 129 apparent from latter equation that despite its simple form, the "Z formula" accounts for the
 130 hydraulic characteristics of the sewer network which, except T (which is usually assumed
 131 constant) all other parameters of eq. (3) are calculated using the simulation model.
 132 Furthermore the concentration of BOD_5 loading was assumed to be invariant during the day,
 133 thus, it can be calculated by dividing the daily mass of BOD_5 with the daily sewage volume.
 134 According to Pomeroy, (1990) values of $Z_i > 7500$ indicate that there are high chances of H_2S

135 formation which could lead to odour and corrosion problems. Eq. (3) can be used for a single
136 pipe, thus we used a modified version of index Z of Pomeroy for a “chain” of pipes n :

$$MZ_c = \sum_{i=1}^n a_i \times Z_i \quad (4)$$

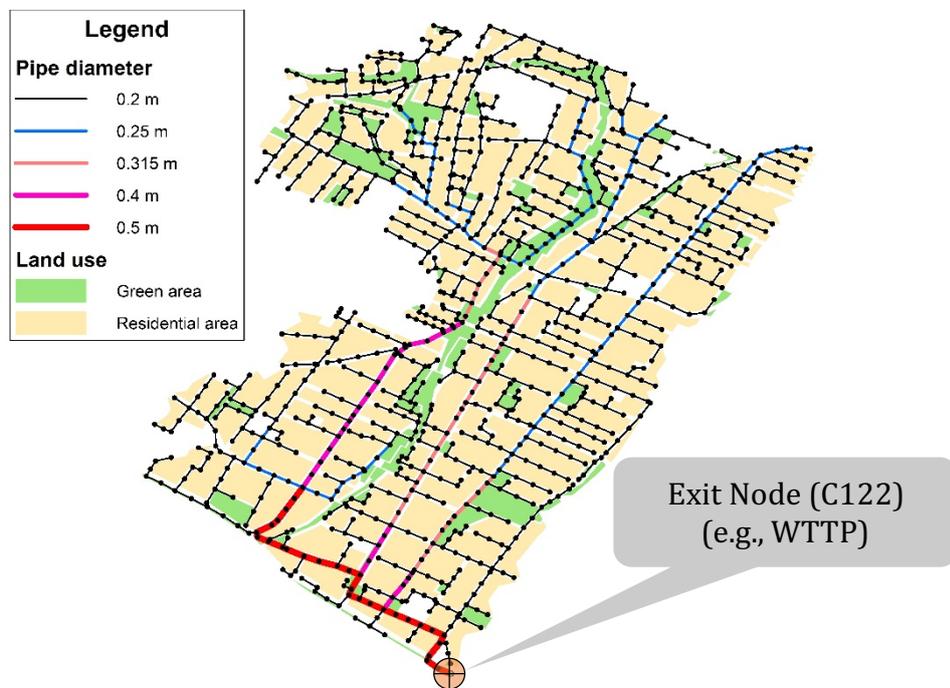
137 Where, a_i are weight coefficients. In this study we use weight values proportional to pipe length
138 using the following formula, $a_i = L_i/L_{tot}$, where, L_i is the length of pipe i , and L_{tot} is the total length
139 of pipes of chain ($i = 1, \dots, n$). It is worth mentioning that literature includes a variety of metrics
140 (Boon, 1995; Hvitved-Jacobsen et al., 2013; Lahav et al., 2006; Marleni et al., 2015), other than
141 Pomeroy's Z , that could be used to quantify (with higher precision) the amount of H_2S in terms
142 of mg/l. Since we performed N model simulations (step II) we have N values of MZ_c for each
143 path and for each green area, therefore we are able to calculate $Q[MZ_c]_x$ which represents the
144 value of the desired quantile x . For example the 75th quantile value indicate that 75% percent
145 of MZ_c are below $Q[MZ_c]_{75}$ value. Through this way we impose an additional reliability criterion
146 for H_2S build-up. Finally, for each green area, among all available paths we select the one with
147 (optimum) minimum $Q[MZ_c]_x$ value. To this point we have located the nodes with minimum
148 $Q[MZ_c]_x$, thus we could fuse it with information regarding the water demand in the areas of
149 interest (green areas). We select as approximate indicator for water demand the area of the
150 park. Similarly, the actual water demand of each area could be more accurately calculated if
151 relevant information was available. It is worth mentioning that the use of multi-criteria analysis
152 allows the inclusion of other metrics regarding other aspects of the network, hence, provides a
153 powerful tool for exploring alternative options and decisions.

154 **3 Case study**

155 **3.1 Description**

156 The methodology is demonstrated in a sewer network designed for the city of Kalyvia Thorikou
157 in Greece (Figure 2). The network has not been constructed yet, although it is foreseen to

158 accommodate an area of 98 ha from which 17 ha are green areas. It is part of a larger
159 engineering project of Saronikos municipality (service 10 - 15 thousand people) which aims at
160 extending the existing sewage network of coastal zone. It is consisted of 1030 pipes of total
161 length ~38 km, while their diameter varies from 0.2 m to 0.5 m. The pipe slope varies from 2‰
162 to 150‰, with an average slope of 35‰. The understudy area can be considered appropriate
163 for testing the proposed methodology, since it is consisted of various network elements and has
164 adequate number of green areas which could benefit from sewer mining practices.
165



166

167

Figure 2: Case study sewer network and land uses - Kalyvia Thorikou, Greece.

168

3.2 Problem setup

169

The design period of the network was assumed, $T = 40$ years, as in the original study of

170

Hydroexigiantiki, the engineering firm which conducted the study of the above network. The

171

design population (E) is adjusted using the compound rate formula $E = E_o \times (1 + \varepsilon)^n$, where, E_o

172

is the current population, ε is the increase rate (assumed 1.5%) and n is the extrapolation year

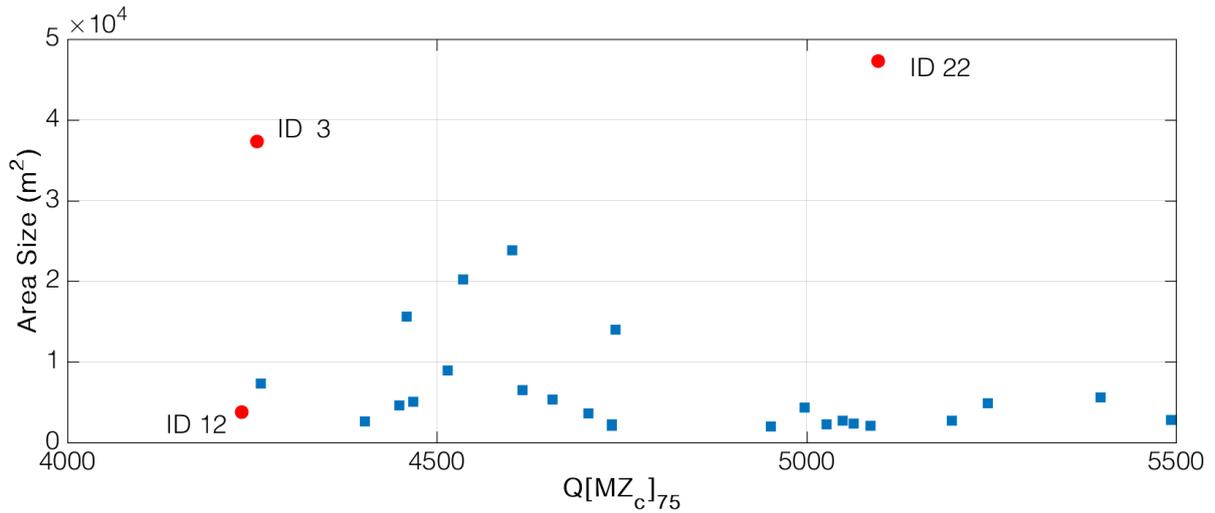
173

($n = 0, \dots, T$). The value of n can be varied in order to assess the performance of the system at

174 different time periods. In this study, q was assumed to be equal to 250 lpd, λ_L was assumed equal
175 to 0.725 for year 0 and 0.85 for year 40. Similarly, λ_s was assumed equal to 0.625 for year 0 and
176 0.65 for year 40. The values of λ_L and λ_s for intermediate years can be calculated using linear
177 interpolation. The value of λ_{DWF} was set equal to 0.2. Finally, we assumed λ_1 and λ_2 as uncertain
178 parameters that follow uniform distribution; i.e., we assumed $\lambda_1 \sim Uniform[0.7, 1.3]$ and $\lambda_2 \sim$
179 $Uniform[0.8, 1]$. As far it concerns parameter n , we employed three scenarios, 0, 20 and 40 years.
180 Also, the mass of BOD₅ was varied using three scenarios 40, 50, and 65 g/(day cap). The
181 maximum allowable number of simulation runs for the MCS step was set equal to 500. The
182 desired quantile x (i.e., reliability level) for the calculation of $Q[MZ_c]_x$ was set to 75%.

183 **4 Results and discussion**

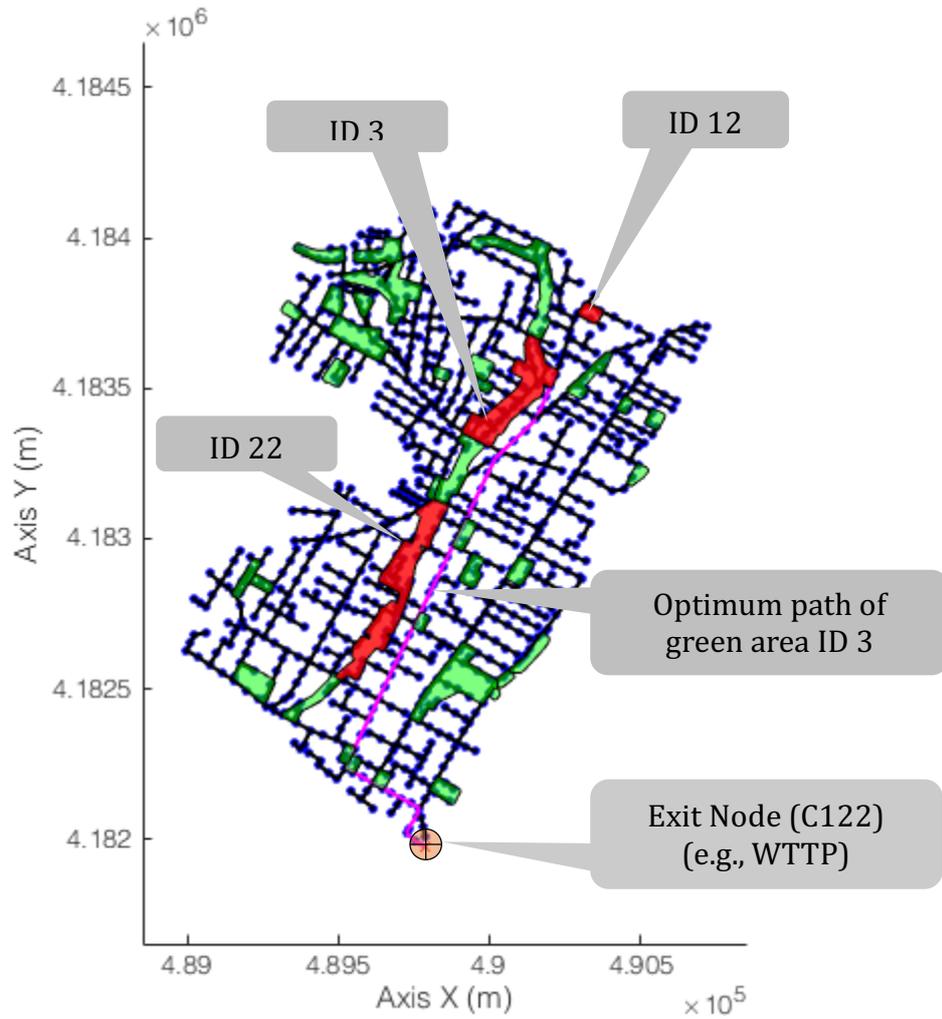
184 Figure 3 illustrates the final result of the post-processing step III in a form of a Pareto front,
185 using as objectives the minimization of modified Z index and the maximization of green area. It
186 is notable that one could also interpret those two objectives as the simultaneously
187 maximization of suitability and benefit from sewer mining practices respectively. The
188 suggested procedure located three potential locations for sewer mining units' placement that
189 optimize both criteria simultaneously, while on the other hand discarded other inferior
190 locations. Additionally, the map depicted in Figure 4 provides a visual summary of all the green
191 areas (green polygons) of the case study, as well as the three areas (red polygons) identified by
192 the proposed methodology since they were suitable for SM placement. Furthermore, in order
193 to visually illustrate the concept of optimum path it presents the selected optimum path
194 (magenta line) for the green area with ID3. This path has the lowest MZ value compared to all
195 other alternative paths of ID3.



196

197

Figure 3: Derived Pareto front based on modified indicator Z (MZ) and green area size.



198

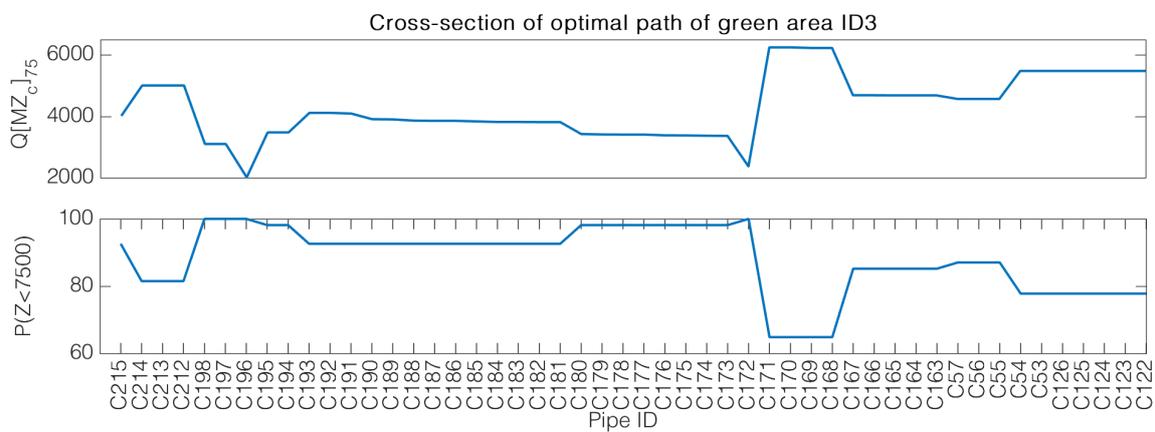
199

Figure 4: Proposed sewer mining locations for Kalyvia Thorikou sewer network

200

Figure 5 depicts the cross-section of optimum path of green area ID3 (magenta line in Figure

201 4). The path starts from pipe C215 which is located close to the green area ID3 and ends to C122
 202 which is linked with the “exit” node of the understudy system. More specifically, the upper panel
 203 of Figure 5 shows the variability of the MZ across that path. Furthermore, the lower panel of
 204 Figure 5 shows the probability of non-exceedance the threshold values $P(Z < 7500)$. It can be
 205 seen that until C171 the system demonstrates high non-exceedance probabilities (~90%), i.e.,
 206 high reliability. After that point the reliability decreases but it is still preserved within
 207 acceptable levels (70-80%).



208
 209 **Figure 5: Cross-section of optimal path of green area ID 3. The upper panel depicts the variation of**
 210 **modified indicator Z (MZ) among longitude profile. The lower panel depicts the probability of non-**
 211 **exceedance of the threshold value of Z = 7500 among the cross-section.**

212 5 Conclusion

213 In order to overcome the engineering challenges imposed by the multiple physical, biological
 214 and chemical processes that take place in a sewer network, we introduced a novel Monte-Carlo
 215 based method for the identification of potential locations for sewer mining units. The proposed
 216 risk-based approach allows to safely plan for SM deployments taking into due consideration
 217 system performance objectives regarding water quantity and quality. As such it can be used to
 218 enhance the decision making process with useful guidelines and insights. More specifically, the
 219 proposed method has been demonstrated though a case study (Kalyvia Thorikou, Greece)
 220 where we focused on identifying optimum locations for sewer mining units subject to the

221 generation (minimize) of hydrogen sulphide (H₂S) and water demand. The results showed that
222 the proposed methodology was able to identify potential locations for sewer mining units'
223 placement while simultaneously taking into consideration the spatial properties of the area as
224 well as the variability and hydraulic characteristics of the sewer network. Future work will
225 focus on improving the proposed framework through the integration of a dynamic simulation
226 model, such as SWMM 5.0 into the computational procedure.

227 **Acknowledgements**

228 The research leading to these results has received funding from the European Union Seventh
229 Framework Programme under grant agreement no. 619039 (ENV.2013.WATER INNO&DEMO-
230 1), for the research project DESSIN 'Demonstrate Ecosystem Services Enabling Innovation in
231 the Water'. The research and its conclusions reflect only the views of the authors and the
232 European Union is not liable for any use that may be made of the information con- tained herein.
233 Also, we would like to sincerely thank Hydroexigiantiki Consulting Engineers for providing the
234 data of the understudy area (Kalyvia Thorikou, Greece). An earlier version of this paper was
235 presented at the 13th IWA Specialized Conference on Small Water and Waste- water Systems
236 (Tsoukalas et al.2016).

237 References

- 238 Andreadakis, A., Mamais, D., Gavalakis, E., 2006. Evaluation of treatment schemes appropriate
239 for wastewater reuse in Greece. *Int. J. Environ. Technol. Manag.* 6, 423–433.
- 240 Bielecki, R., Schremmer, H., 1987. Biogene Schwefelsäure-Korrosion in teilgefüllten
241 Abwasserkanälen. na.
- 242 Boon, A.G., 1995. Septicity in sewers: causes, consequences and containment. *Water Sci.*
243 *Technol.* 31, 237–253.
- 244 Hadzihalilovic, V., 2009. Sewer mining for golf course irrigation. *J. Aust. Water Assoc.* 36, 168–
245 71.
- 246 Hvitved-Jacobsen, T., Vollertsen, J., Nielsen, A.H., 2013. Sewer processes: microbial and chemical
247 process engineering of sewer networks. CRC press.
- 248 Koutsoyiannis, D., 2011. Design of Urban Sewer Networks, 4th ed. National Technical University
249 of Athens, Athens.
- 250 Lahav, O., Sagiv, A., Friedler, E., 2006. A different approach for predicting H₂S(g) emission rates
251 in gravity sewers. *Water Res.* 40, 259–266. doi:10.1016/j.watres.2005.10.026
- 252 Liu, S., Butler, D., Memon, F.A., Makropoulos, C., Avery, L., Jefferson, B., 2010. Impacts of
253 residence time during storage on potential of water saving for grey water recycling system.
254 *Water Res.* 44, 267–277.
- 255 Makropoulos, C.K., Butler, D., 2010. Distributed water infrastructure for sustainable
256 communities. *Water Resour. Manag.* 24, 2795–2816. doi:10.1007/s11269-010-9580-5
- 257 Makropoulos, C., Rozos, E., Tsoukalas, I., Plevri, A., Karakatsanis, G., Karagiannidis, L., Makri, E.,
258 Lioumis, C., Noutsopoulos, C., Mamais, D., Rippis, C. & Lytras, E. Sewer-mining: a water
259 reuse option supporting circular economy, public service provision and entrepreneurship.
260 *J. Environ. Manage.* doi: 10.1016/j.jenvman.2017.07.026.
- 261 Marleni, N., Gray, S., Sharma, A., Burn, S., Muttill, N., 2012. Impact of water source management

262 practices in residential areas on sewer networks - A review. *Water Sci. Technol.* 65, 624–
263 642. doi:10.2166/wst.2012.902

264 Marleni, N., Park, K., Lee, T., Navaratna, D., Shu, L., Jegatheesan, V., Pham, N., Feliciano, A., 2015.
265 A methodology for simulating hydrogen sulphide generation in sewer network using EPA
266 SWMM. *Desalin. Water Treat.* 54, 1308–1317. doi:10.1080/19443994.2014.922899

267 Pomeroy, R.D., 1990. *The Problem of Hydrogen Sulphide in Sewers*. Clay Pipe Dev. Assoc. Ltd.,
268 London, 2 nd Ed. by A. G. Boon), 1990, 24.

269 Sydney Water, 2009. *Recycled water in the Sydney region - sewer mining schemes*.

270 Sydney Water, 2008. *Sewer Mining. How to Establish a Sewer Mining Operation* 14.

271 Tsoukalas, I., Makropoulos, C. & Michas, S. A Monte-Carlo based method for the identification of
272 potential sewer mining locations. *Proceedings of the 13th Small Water and Wastewater*
273 *Systems (SWWS) Conference*, 14–16 September 2016, Athens, Greece.

274